

# GEOTECHNICAL CHARACTERISTICS OF OXISOL B HORIZONS

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The opportunity to determine a few geotechnical characteristics of samples taken from B horizons of Oxisol pedons thoroughly studied by soil scientists seemed to the authors to be a valid exercise. The plan of study included the use of new testing techniques and the engineering soil classification system developed in Brazil by Nogami and Villibor called MCT (miniature compacted tropical) classification.

## Materials and Methods

From the 23 pedons described and collected for the VIIIth International Soil Classification Workshop (ISCW), 9 of them were selected for the realization of this study.

| Sample | Pedon         | Horizon | Soil Taxonomy    |
|--------|---------------|---------|------------------|
| P1     | ISCW 1        | Bo2     | Typic Eutrorthox |
| P4     | ISCW 4        | Bo2     | Typic Hapludox   |
| P7     | ISCW 7        | Bo2     | Typic Acrudox    |
| P11    | ISCW 11       | Bo3     | Rhodic Hapludox  |
| P13    | ISCW 13       | Bo2     | Acrudox          |
| P18    | ISCW 18       | Bo      | Typic Acrudox    |
| P20    | ISCW 20       | Bo2     | Rhodic Eustrudox |
| P22    | ISCW 22       | Bo1     | Typic Acrorthox  |
| Extra  | ISCW Extra II | Bo1     | Typic Acrorthox  |

In Table 1 are the comparative properties of soil groups obtained in miniature compacted specimens (MCT). The lateritic engineering behavior is distinguished from the nonlateritic behavior. The method was developed for soils of grain-size less than 2 mm.

The mini-CBR (California bearing ratio miniaturized) is a penetration test (piston of 1.6 cm diameter) of a sample that has been compacted with the mini-MCV rammer, at a constant

compaction effort of 10 blows. Specimens were not soaked, and a surcharge weight of 490 g was placed on the top surface during the penetration.

## Results

Determinations made at EMBRAPA are in Table 2. The parameters from mini-MCV tests, together with grain-size data, liquid limit and plasticity index (classes according to U.S. Highway Research Board Unified Soil Classification System) and miniature compacted specimens (MCT) are presented in Table 3. These tests were conducted at the Soil Mechanics Laboratory of COPPE/UFRJ. Test results of mini-CBR are shown in Table 4, and the graphs of compaction curves ( $\gamma_d, w$ ) and CBR curves (mini-CBR,  $w$ ) are in Figure 1. In Figure 2 were also plotted the points representative of the nine samples.

## Discussion

All soil samples plot on the lateritic behavior part of the chart for classification purposes (Fig. 2). Those of high immersion loss are near the borderline of classes LG' and NG'. Soil samples P4 (Typic Hapludox) and P22 (Typic Acrorthox) have immersion losses of 10 percent and zero respectively. Sample P13, an Acrudox, is more sandy than the previous ones and has an immersion loss of 100 percent, which means lack of strong cementation when saturated.

The usual testing procedures of SNLCS laboratories were applied to determine grain-size distribution, silt to clay ratio, and CEC  $k_i$  and  $k_r$ . The mini-MCV (moisture condition value) test is described by Nogami and Villibor (1981, 1985). A sample of soil weighing about 200 g is compacted in miniaturized equipment similar to that developed by Parsons and Boden (1976). The rammer weight is 2740 g, the height of drop 305 mm, and the diameter of mold 50 mm (Fig. 3).

The difference to the traditional Proctor compaction test much used in earthworks throughout the world is that instead of applying a standardized compactive effort to the moistened mass of soil, successive blows are applied while the decreasing height of the compacted soil sample is measured. This procedure is repeated for different water contents. The MCV value is defined:

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**Table 1. Comparative Properties of Soil Groups**

|  |           |                     |                           |     |       |     |                        |      |     |
|--|-----------|---------------------|---------------------------|-----|-------|-----|------------------------|------|-----|
| A = sand (Arena)<br>A' = sandy<br>G = clay (Argilla)<br>G' = clayey<br>S = silt<br>S' = silty<br>k = kaolinitic<br>m = micaceous<br>q = quartzose<br>h = high<br>l = low<br>m = medium<br>v = very<br>/ = to | Mini-CBR* | Class               | Nonlateritic (N) behavior |     |       |     | Lateritic (L) behavior |      |     |
|  |           | Group               | NA                        | NA' | NS'   | NG' | LA                     | LA'  | LG' |
|  |           | Typical grain size  | A                         | S'A | S(km) | G   | A                      | G'A  | G   |
|  |           |                     | S'A                       | G'A | A'S   | A'G |                        | A'G  | A'G |
|  |           |                     | S(q)                      |     | G'S   | S'G |                        |      |     |
|  |           | Soaked              | h/m                       | h/m | m/h   | h   | h                      | h/vh | h   |
|  |           | Decrease by soaking | m/l                       | l   | h     | h   | l                      | l    | l   |
|  |           | Swelling *          | l                         | l   | h     | h/m | l                      | l    | l   |
|  |           | Shrinkage *         | l/m                       | l/m | m     | h/m | l                      | l/m  | m/h |
|  |           | Permeability*       | m/h                       | l   | m/l   | l/m | m/l                    | l    | l   |
| Plasticity   | l/NP      | m/NP                | m/h                       | h   | NP/l  | l/m | m/h                    |      |     |

\* Specimens compacted at optimum of standard compactive effort.

**Table 2. Soil Parameters and Geotechnical Classification of Oxisols Studied**

| Sample   | % Pass.<br>200 | WL/ $I_p$ | $c'$ | $d'$ | $P_i$ | $e'$ | Classification |      |         |
|----------|----------------|-----------|------|------|-------|------|----------------|------|---------|
|          |                |           |      |      |       |      | HRB            | USCS | MCT     |
| P1<br>64 | <50            | 58        | 1.5  | 77   | 0     | 0.64 | A-7-5          | SM   | LA'/LG' |
|          |                | 14        |      |      |       |      |                |      | LG'     |
| P4       | >50            | 21<br>45  | 1.7  | 67   | 10    | 0.74 | A-7-5          | MH   | LG'     |
| P7       | >50            | 12<br>39  | 2.1  | 27   | 80    | 1.13 | A-7-5          | ML   | LG'     |
| P11      | >50            | 9<br>47   | 2.6  | 15   | 30    | 1.18 | A-4            | ML   | LG'     |
| P13      | <50            | 11<br>46  | 0.6  | 10   | 60    | 1.37 | A-2-7          | SM   | LA      |
| P18      | >50            | 10<br>32  | 1.8  | 57   | 100   | 1.11 | A-5            | ML   | LG'     |
| P20      | >50            | 11<br>36  | 2.0  | 32   | 100   | 1.18 | A-6            | CL   | LG'     |
| P22      | >50            | NP<br>52  | 2.0  | 40   | 0     | 0.79 | A-4            | ML   | LG'     |
| Extra II | >50            | 28        | 2.2  | 44   | 20    | 0.87 | A-7-6          | CH   | LG'     |

Note: WL = liquid limit;  $I_p$  = plasticity index;  $c'$ ,  $d'$ ,  $P_i$ ,  $e'$  = parameters defined in text.

**Table 3. Testing Data of Samples from EMBRAPA**

| Sample   | CEC | $k_i$ | $k_r$ | Silt/Clay (%) | Grain size (%) |      |      |
|----------|-----|-------|-------|---------------|----------------|------|------|
|          |     |       |       |               | Sand           | Silt | Clay |
| P1       | 2.9 | 1.40  | 1.22  | 15            | 53             | 6    | 41   |
| P4       | 2.8 | 1.78  | 1.39  | 13            | 14             | 10   | 76   |
| P7       | 1.3 | 0.71  | 0.41  | 18            | 10             | 14   | 76   |
| P11      | 1.3 | 0.37  | 0.27  | 11            | 3              | 10   | 87   |
| P13      | 1.3 | 1.82  | 1.28  | 39            | 75             | 7    | 18   |
| P18      | 1.8 | 0.91  | 0.69  | 10            | 12             | 8    | 80   |
| P20      | 2.4 | 0.71  | 0.55  | 9             | 13             | 7    | 80   |
| P22      | 4.9 | 1.53  | 1.20  | 38            | 34             | 18   | 48   |
| Extra II | 2.8 | 0.43  | 0.25  | 18            | 11             | 14   | 75   |

Note: Sand 2-0.05 mm; silt 0.05-0.002 mm; clay <0.002 mm.

**Table 4. Compaction Parameters ( $\gamma_d$ ,  $w$ ) and Mini-CBR Values**

| Sample   |            | Specimen |       |       |       |       |       |       |       |       |       | Wo   |
|----------|------------|----------|-------|-------|-------|-------|-------|-------|-------|-------|-------|------|
|          |            | 1        | 2     | 3     | 4     | 5     | 6     | 7     | 8     | 9     | 10    |      |
| P1       | w%         | 16.91    | 17.21 | 19.28 | 22.06 | 22.75 | 12.38 | 14.24 | 16.44 | 17.61 | 19.92 | 18   |
|          | MCBR       | 25       | 18    | 7     | 6     | 3     | 31    | 26    | 25    | 17    | 10    | 16   |
|          | $\gamma_d$ | 17.51    | 17.64 | 16.91 | 16.52 | 16.10 | 15.23 | 15.87 | 17.32 | 17.84 | 17.07 | 17.8 |
| P4       | w%         | 26.06    | 27.88 | 30.65 | 31.49 | 35.35 | 25.64 | 28.44 | 30.39 | 31.80 | 34.13 | 28.5 |
|          | MCBR       | 20       | 17    | 9     | 8     | 3     | 20    | 16    | 13    | 9     | 5     | 16   |
|          | $\gamma_d$ |          |       |       |       |       | 14.24 | 14.73 | 14.37 | 14.07 | 13.59 | 14.7 |
| P7       | w%         | 24.71    | 26.14 | 27.80 | 30.51 | 32.23 | 22.90 | 25.05 | 26.66 | 29.38 | 31.29 | 27   |
|          | MCBR       | 20       | 16    | 9     | 5     | 3     | 22    | 18    | 11    | 7     | 4     | 10   |
|          | $\gamma_d$ |          |       |       |       |       | 15.66 | 16.35 | 16.55 | 15.74 | 15.20 | 16.6 |
| P11      | w%         | 27.01    | 31.29 | 32.20 | 33.97 | 36.56 | 23.18 | 26.34 | 29.47 | 29.72 | 34.47 | 30   |
|          | MCBR       | 21       | 14    | 10    | 7     | 4     | 35    | 27    | 21    | 19    | 9     | 20   |
|          | $\gamma_d$ |          |       |       |       |       | 11.99 | 13.14 | 14.16 | 13.98 | 13.71 | 14.3 |
| P13      | w%         | 8.87     | 10.91 | 13.36 | 15.05 | 17.60 | 9.00  | 10.95 | 12.39 | 14.34 | 17.35 | 11   |
|          | MCBR       | 25       | 20    | 16    | 7     | 3     | 33    | 27    | 14    | 5     | 2     | 25   |
|          | $\gamma_d$ |          |       |       |       |       | 18.78 | 18.99 | 18.93 | 18.17 | 17.19 | 19.0 |
| P18      | w%         | 28.09    | 28.95 | 30.79 | 33.01 | 34.55 | 20.43 | 21.94 | 23.71 | 25.54 | 26.47 | 28   |
|          | MCBR       | 14       | 15    | 8     | 4     | 3     | 35    | 35    | 25    | 21    | 18    | 16   |
|          | $\gamma_d$ |          |       |       |       |       | 12.26 | 12.28 | 12.81 | 13.77 | 14.42 | 14.7 |
| P20      | w%         | 26.84    | 28.94 | 31.25 | 33.73 | 35.33 | 16.61 | 19.92 | 20.11 | 22.49 | 24.71 | 26   |
|          | MCBR       | 18       | 15    | 9     | 6     | 3     | 38    | 34    | 33    | 29    | 21    | 20   |
|          | $\gamma_d$ |          |       |       |       |       | 12.06 | 11.98 | 12.04 | 12.22 | 13.48 | 14.1 |
| P22      | w%         | 15.84    | 17.58 | 20.16 | 21.61 | 24.00 | 14.37 | 16.83 | 18.97 | 18.86 | 22.64 | 19   |
|          | MCBR       | 23       | 16    | 14    | 8     | 4     | 25    | 21    | 20    | 19    | 7     | 20   |
|          | $\gamma_d$ |          |       |       |       |       | 15.41 | 16.28 | 16.68 | 16.54 | 16.03 | 16.6 |
| Extra II | w%         | 26.01    | 28.14 | 30.08 | 32.08 | 34.28 | 20.08 | 22.45 | 23.47 | 25.65 | 27.49 | 28   |
|          | MCBR       | 10       | 16    | 9     | 5     | 2     | 36    | 20    | 22    | 20    | 19    | 16   |
|          | $\gamma_d$ |          |       |       |       |       | 13.23 | 13.89 | 14.27 | 15.16 | 15.58 | 15.6 |



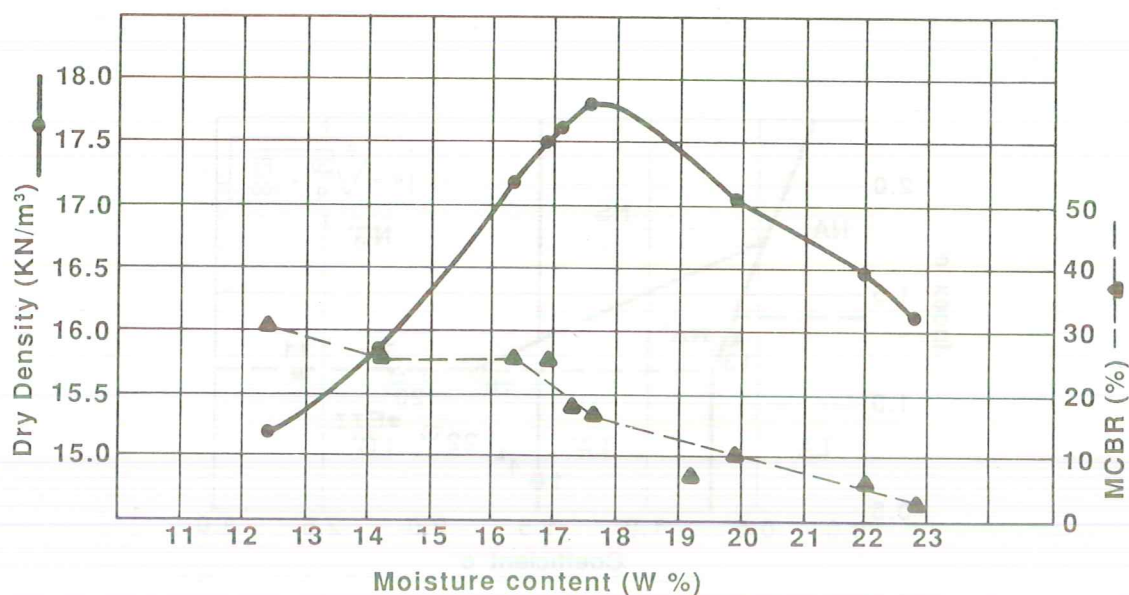


Fig. 1. Example of compaction and mini-CBR curves (sample P20).

$$\text{mini-MCV} = 10 \log_{10}(B_n) \quad (1)$$

$B_n$  is the intersection of mini-MCV curves shown in Figure 4 with a straight line corresponding to  $a_n$  equal to 2 mm.

$$a_n = A_n - A_{4n} \quad (2)$$

$A_n$  is the height for  $n$  blows and  $A_{4n}$  for  $4n$  blows.

The parameter  $c'$  is picked up from the mini-MCV curves. After compaction the soil sample is partially extruded from the mold so that a part 1 cm long becomes salient. Then the mold with the extruded sample is flooded until the water level comes at least 1 cm above the mold. The loss of weight by immersion ( $P_i$ ) is determined. Its plot is shown in the same figure. From the compaction curves, where dry density of compacted soil is plotted against moisture content the  $d'$  parameter (average slope of 12-blows curve in the dry side) is determined. To enter the classification chart (Fig. 2) two parameters are needed:  $c'$  and  $e'$ . The latter is defined as:

$$e' = \left( \frac{P_i}{100} + \frac{20}{d'} \right)^{1/3} \quad (3)$$

It can be observed that soils having a high value of  $P_i$  (immersion loss) are Acrox ones; they are old soils with little CEC. However, data are not enough for a tentative correlation of  $P_i$  with CEC (or with  $k_i$  or  $k_c$ ).

Soil  $P_i$  is at the borderline LA/LG'. It has approximately the same amounts of sand and clay fractions.

With data from Tables 2 and 3 some correlations were tested:

1. Parameter  $c'$

$$c' = 2.6023 - 0.0475 (\text{silt/clay, \%}); r = 0.773 \quad (4)$$

$$c' = 2.3414 - 0.0203 (\text{sand, \%}); r = 0.882 \quad (5)$$

$$c' = 0.5397 + 0.0200 (\text{clay, \%}); r = 0.836 \quad (6)$$

$$c' = 2.6577 - 0.7681 (k_i); r = 0.779 \quad (7)$$

$$c' = 2.5321 - 0.8663 (k_c); r = 0.720 \quad (8)$$

These correlations are reasonably good.

2. Parameter  $d'$   
 $d' = -13.85 + 26.49 (\text{CEC}); r = 0.790 \quad (9)$

3. Parameter  $e'$   
 $e' = 0.2284 + 0.0103 (\text{clay, \%}); r = 0.779 \quad (10)$

$$e' = 1.29 - 0.36 (k_i); r = 0.750 \quad (11)$$

$$e' = 1.23 - 0.37 (k_c); r = 0.770 \quad (12)$$

$$e' = 1.3713 - 0.1150 (\text{CEC}); r = 0.729 \quad (13)$$

The parameter  $P_i$  does not correlate with the variables considered. The  $d'$  value correlates only with the CEC. But as  $e'$  is an empirical function of  $P_i$  and  $d'$ , some kind of correlation might be tried. More data are needed.

The classical soil classification systems used in Brazil for engineering purposes—HRB (now TRB) or AASHTO classification and the USCS—do not detect the so-called lateritic behavior. One soil may be classified in a weak category as subgrade material, yet may behave very well because of its structure and the cementing effect of iron oxides. CBR values are sometimes surprisingly high for what could be expected from a clayey soil, or a fine lateritic sand, according to above mentioned classifications. Therefore, the attempt to translate pedological data into engineering information cannot be accomplished by performing a few extra tests such as the Atterberg limits and the compaction tests. What should be done

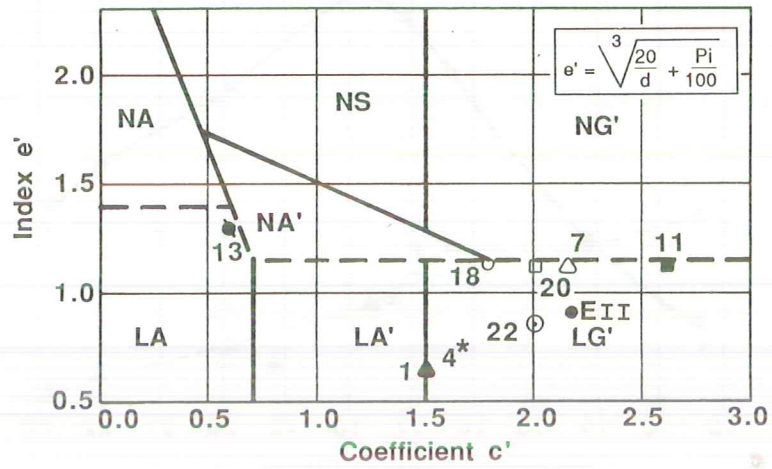


Fig. 2. Chart for classification purpose (MCT).

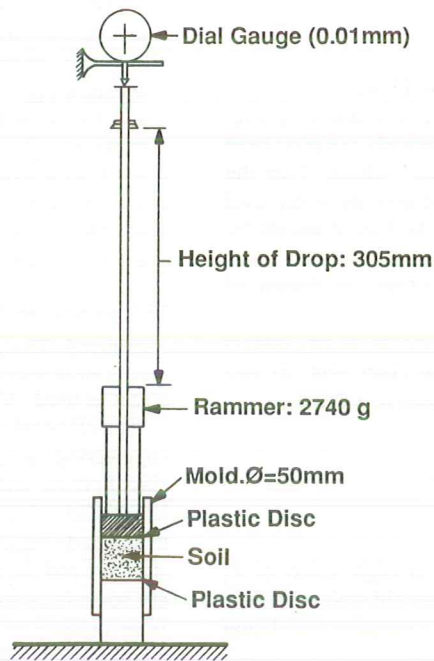


Fig. 3. Schematic Mini-MCV compaction apparatus.

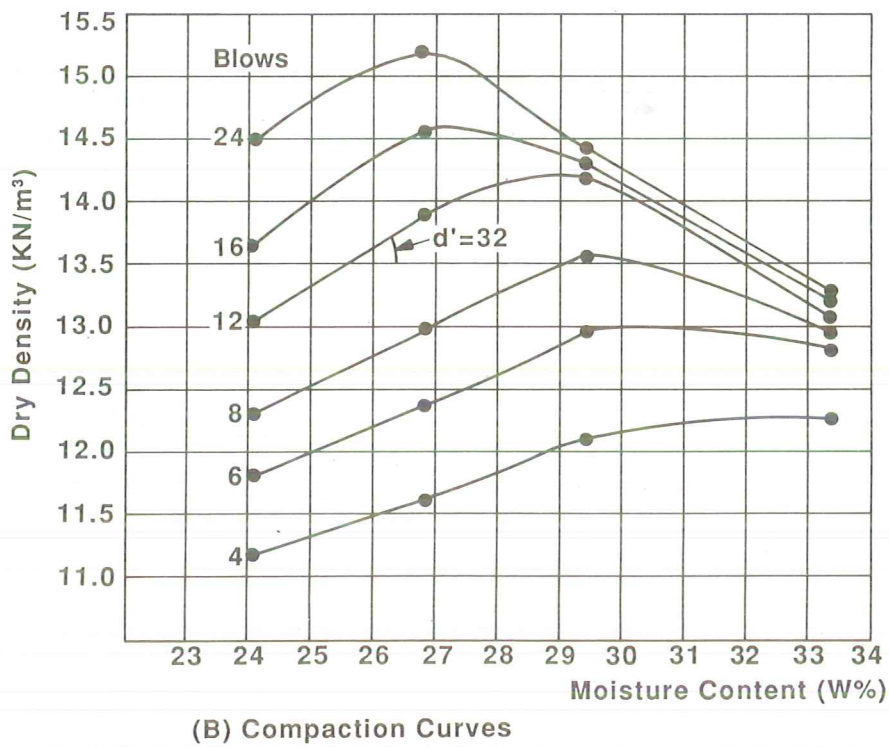
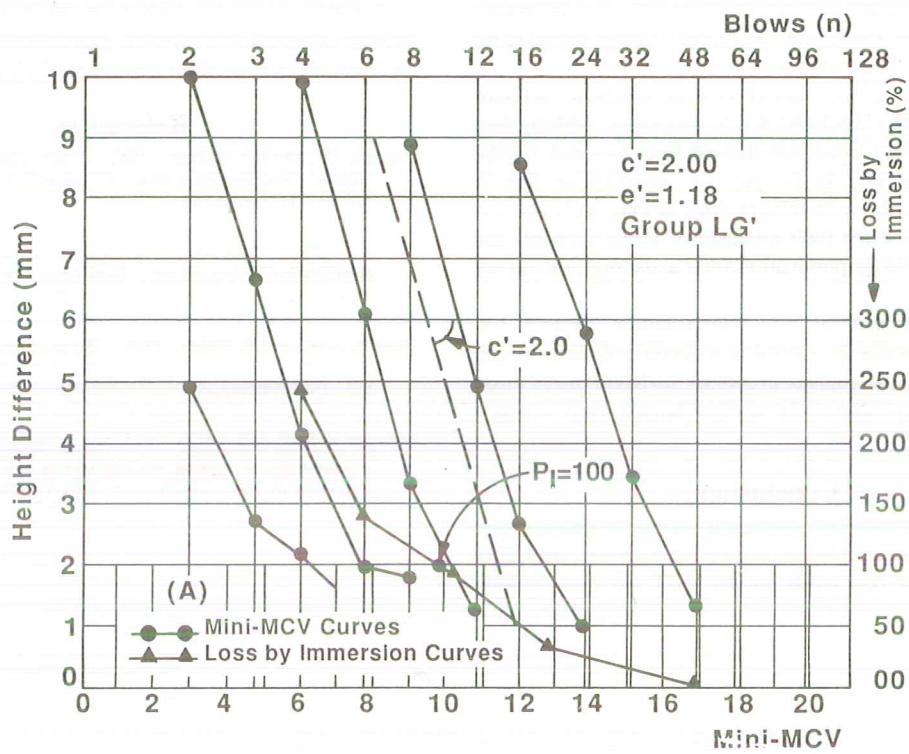


Fig. 4. Graphic representation of test results.

is either to correlate soil series with performance of engineering constructions or to perform more significant testing reflecting properties that explain engineering behavior. For pavement design of low cost roads, the MCT classification seems to be quite adequate for Brazilian soils.

Each purpose to be served best demands a different classification. The classification is a contrivance made by men to suit their purposes, not a truth that can be discovered. On the other hand taxonomy is the part of classification that is concerned primarily with relationships among soils and the factors responsible for their character. These concepts are clearly presented in the publication *Soil Taxonomy* (Soil Survey Staff 1975).

Therefore, a general soil classification system for engineering purposes developed in a country of climates and weathering and pedological processes different from those prevalent in Brazil cannot be applied blindly as if it was a taxonomic system.

### Conclusion

The nine samples taken from the B horizon of Oxisols were evaluated more accurately for engineering purposes, especially for paving of low cost roads, by the MCT soil classification

system than by simply determining the classes they belong to in the Unified and HRB soil classification systems.

A strong possibility exists for determining relationships between physical and chemical characteristics of Oxisols and the parameters needed for geotechnical classification.

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